

Client	FREYSSINET - ETHAD TOWER EB	DATE	-08/02/2009
Bearing 3 - P4PT3 - Permutation - QTY 1 No			
<b>BEARING DIMENSIONS</b>			
a	Overall Width	mm	480.0
b	Overall Length	mm	900.0
c	Edge Cover of Elastomer	mm	4.0
$a_e$	Effective Width	mm	472.0
$b_e$	Effective Length	mm	892.0
$n_i$	Number of inner Elastomer Layers		4.0
$t_i$	Thickness of an inner Layer	mm	15.0
$n_o$	No. of Outer Cover Layer		2.0
$t'$	Thickness of outer Cover Layer -	mm	3.8
$n_r$	No. of reinforcing plates		5.0
t	Thickness of reinforcing plates	mm	6.1
$h_e$	Actual thickness of Elastomer	mm	60.0
h	Overall thickness of Bearing	mm	98.0
<b>ELASTOMER PROPERTIES</b>			
H	Nominal Hardness acc to EN 1337	IRHD	60.0
$G_l$	long term shear modulus	N/mm <sup>2</sup>	0.90
$G_s$	short term shear modulus / dynamic	N/mm <sup>2</sup>	1.80
$E_b$	bulk modulus	N/mm <sup>2</sup>	2000.0
Cc	Compression Stiffness required as per design	kN/mm	800.0
Cs	Shear Stiffness required as per design	kN/mm	2.0
<b>DESIGN CALCULATIONS</b>			
<b>DESIGN LOADS</b>			
Vertical KN	$V_p$	permanent	3742
	$V_l$	live /sesmic	806
	V	design load effect = DD x 1.2 + LL x 1.6	5780
	$V_{min}$	min design load	5780
Horizontal KN	$H_{pb}$	permanent // to length b - Transverse Force	0
	$H_{lb}$	live /sesmic //to length b - Transverse Force	0
	$H_{pa}$	permanent // to width a - Longitudinal Force	0
	$H_{la}$	live /sesmic //to width a - Longitudinal Force	0
	$H_{vm}$	acting with $V_{min}$	0
<b>DISPLACEMENTS &amp; ROTATION</b>			
Disp (mm)	$\delta_{pa}$	permanent disp // to width a - Longitudinal	0.0000
	$\delta_{pb}$	permanent disp // to length b - Transverse	0.0000
Rotations (rad)	$\alpha_c$	const tolerance	0.0010
	$\alpha_{pa}$	longitudinal perm rotation	0.0020
	$\alpha_{pb}$	transverse perm rotation	0.0010
	$\alpha'_{a}$	$\alpha_{pa} + \alpha_c$	0.0030
	$\alpha'_{b}$	$\alpha_{pb} + \alpha_c$	0.0020
	$\alpha_{la}$	logitudinal live/sesmic rotation	0.0020
	$\alpha_{lb}$	transverse live/sesmic rotation	0.0020
	$\alpha_{ma}$	logitudinal maximum rotation	0.0050
	$\alpha_{mb}$	transverse maximum rotation	0.0040
<b>SHAPE FACTOR</b>			
S	Shape Factor	10.289	10.289
A <sub>1</sub>	Effective plan area (mm <sup>2</sup> )	421024	421024
I <sub>p</sub>	Force Free Parameter (mm)	2728	2728
<b>LIVE LOAD DISPLACEMENTS</b>			
Kd	dynamic stiffness (KN/mm)	12.631	12.631
Ks	static shear stiffness(KN/mm)	6.32	6.32
$\delta_{la}$	live/sesmic disp // width a (mm) - Longitudinal	0.00	0.00
$\delta_{lb}$	live/sesmic disp // length b (mm) - Transverse	0.00	0.0
<b>DESIGN LOAD DISPLACEMENTS</b>			
$\delta_a$	design load disp // width a (mm) - Longitudinal	0.0	0.0
$\delta_b$	design load disp // length b (mm) - Transverse	0.0	0.0
<b>SHEAR STRAIN</b>			
$\epsilon_{qa}$	Design load strain // width a - Longitudinal	0.00	0.00
$\epsilon_{qb}$	Design load strain // length b - Transverse	0.00	0.00
$\epsilon_q$	resultant shear strain	0.00	0.00
$\epsilon_{pv}$	permissible value	0.7	2.0
CHECKS $\epsilon_{pv} > \epsilon_q$		PASS	PASS

STRAIN DUE TO COMPRESSIVE LOAD		SLS	ULS					
Ar	reduced A1 (mm <sup>2</sup> )	421024	421024					
ε cl	Strain due to live load	0.16	0.34					
ε cp	Strain due to Permanent Load	1.44	1.44					
ε cd	Strain due to design load - long term G		2.83					
STRAIN DUE TO ANGULAR ROTATION		SLS	ULS					
ε al	Strain due to live load	0.91	0.91					
ε cp	Strain due to Permanent Load	1.00	1.00					
MAXIMUM DESIGN STRAIN		SLS	ULS					
K <sub>L1</sub>	type loading factor for live loads - Safety factor	1.0						
K <sub>Lp</sub>	type loading factor for all others - Safety factor	1.0						
K <sub>L</sub>	type loading factor		1.0					
Y <sub>m</sub>	partial safety factor	1.4	1.15					
ε u	maximum permissible value	7.0	7.0					
ε t	sum of strains	3.50	3.83					
ε pv	permissible value	5.0	6.09					
CHECK ε t < ε pv		PASS	PASS					
ROTATION LIMITATION		SLS	ULS					
K <sub>r</sub>	rotation factor	3.0	3.0					
z <sub>l</sub>	live load def of an individual inner layer (mm)	0.0	0.1					
z <sub>p</sub>	perm load def of an individual inner layer (mm)	0.3	0.3					
Σ z	total vertical deflection (mm)	2.1	2.7					
z min	min vertical def which allows rotation (mm)	1.98	1.98					
CHECK Σ z > z min		PASS	PASS					
VERTICAL DEFLECTION LIMITATION		SLS	ULS					
Σ z <sub>l</sub>	vertical deflection under live loads (mm)	0.18	-					
z Max	max vertical deflection under live load (mm)	1.01	-					
CHECK z max > Σ z <sub>l</sub>		PASS	-					
FIXING OF BEARING		SLS	ULS					
σ m	average comp stress due to V min (N/mm <sup>2</sup> )	13.73	17.44					
σ c <sub>dmin</sub>	min comp stress due to perm load ( N/mm <sup>2</sup> )	8.89	8.89					
σ pv	permissible value	3.00	3.00					
μ <sub>er</sub>	friction coefficient between concrete and rubber	0.6	0.6					
μ <sub>ec</sub>	friction coefficient between concrete & checkered plates	0.5	0.5					
H <sub>a</sub>	design horizontal load // to width a (kN) -Longitudinal	0	0					
H <sub>b</sub>	design horizontal load // to length b (kN) - Transverse	0	0					
H	resultant horizontal force (kN)	0	0					
H <sub>r</sub>	resistance force due to μe (kN)	2245	2245					
H <sub>rc</sub>	resistance force due to pec (kN)	1871	1871					
CHECK σ c <sub>dmin</sub> > σ pv		PASS	PASS					
CHECK H <sub>r</sub> > H		PASS	PASS					
CHECK H <sub>rc</sub> > H ( Required only in case of Top/Bottom Checker plate )		PASS	PASS					
BUCKLING STABILITY		SLS	ULS					
σ d	average comp stress due to V min (N/mm <sup>2</sup> )	13.73	17.44					
σ pv	permissible value (( this calculation uses long term G )	48.56	48.56					
CHECK σ pv > σ d		PASS	PASS					
STEEL PLATE THICKNESS		SLS	ULS					
f <sub>y</sub>	yield stress of the steel ( N/mm <sup>2</sup> )	220	220					
K <sub>p</sub>	Stress correction factor	1.3	1.3					
K <sub>h</sub>	induced tensile stresses factor	1.3	1.3					
Y <sub>m</sub>	partial safety factor	1.5	1.5					
t <sub>s1</sub>	minimum thickness of steel plate (mm)	4.7	6.0					
t <sub>s2</sub>	minimum thickness of steel plate (mm)	2.0	2.0					
t	Thickness of reinforcing plates (mm)	6.1	6.1					
CHECK t > ts1 and t > ts2		PASS	PASS					

SI No	Bearing Size		Elastomer Thickness	No of Elastomer	MS Shim Thickness	No of Shims	Cover Layer	Net Height
	Length	Width	mm		mm		mm	mm
1	480	900	14	4	6	5	4	94
2	480	900	15	4	6	5	4	98
3	485	900	14	4	6	5	4	94
4	485	900	15	4	6	5	4	98
5	490	900	14	4	6	5	4	94
6	490	900	15	4	6	5	4	98
7	480	905	14	4	6	5	4	94
8	480	905	15	4	6	5	4	98
9	485	905	14	4	6	5	4	94
10	485	905	15	4	6	5	4	98
11	490	905	14	4	6	5	4	94
12	490	905	15	4	6	5	4	98
13	480	910	14	4	6	5	4	94
14	480	910	15	4	6	5	4	98
15	485	910	14	4	6	5	4	94
16	485	910	15	4	6	5	4	98
17	490	910	15	4	6	5	4	98
18	480	915	14	4	6	5	4	94
19	480	915	15	4	6	5	4	98
20	485	915	14	4	6	5	4	94
21	485	915	15	4	6	5	4	98
22	490	915	15	4	6	5	4	98
23	480	920	14	4	6	5	4	94
24	480	920	15	4	6	5	4	98
25	485	920	15	4	6	5	4	98
26	490	920	15	4	6	5	4	98
27	480	925	14	4	6	5	4	94
28	480	925	15	4	6	5	4	98
29	485	925	15	4	6	5	4	98
30	490	925	15	4	6	5	4	98
31	480	930	15	4	6	5	4	98
32	485	930	15	4	6	5	4	98
33	490	930	15	4	6	5	4	98
34	480	935	15	4	6	5	4	98
35	485	935	15	4	6	5	4	98
36	490	935	15	4	6	5	4	98
37	480	940	15	4	6	5	4	98
38	485	940	15	4	6	5	4	98
39	490	940	15	4	6	5	4	98
40	480	945	15	4	6	5	4	98
41	485	945	15	4	6	5	4	98
42	490	945	15	4	6	5	4	98
43	480	950	15	4	6	5	4	98
44	485	950	15	4	6	5	4	98
45	490	950	15	4	6	5	4	98
46	480	955	15	4	6	5	4	98
47	485	955	15	4	6	5	4	98
48	490	955	15	4	6	5	4	98
49	480	960	15	4	6	5	4	98
50	485	960	15	4	6	5	4	98
51	490	960	15	4	6	5	4	98
52	480	965	15	4	6	5	4	98
53	485	965	15	4	6	5	4	98
54	490	965	15	4	6	5	4	98
55	480	970	15	4	6	5	4	98
56	485	970	15	4	6	5	4	98
57	490	970	15	4	6	5	4	98
58	480	975	15	4	6	5	4	98
59	485	975	15	4	6	5	4	98
60	490	975	15	4	6	5	4	98
61	480	980	15	4	6	5	4	98
62	485	980	15	4	6	5	4	98
63	490	980	15	4	6	5	4	98
64	480	985	15	4	6	5	4	98

